

Tailings Dam Monitoring to Reduce Risk of Failures

Prof David Williams

D.Williamsi@uq.edu.au

Director Geotechnical Engineering Centre
School of Civil Engineering
The University of Queensland

7 November 2019

Constraints Under Which Surface TSFs Must Operate

- Climatic, topographic and seismic settings
- Nature of tailings, particularly sulfidic minerals and presence of clay minerals
- Tailings production rate and % solids on deposition
- Dam foundation conditions and borrow materials
- Need to maximise tailings settled dry density and strength
- Mitigating risk of a (sudden) spill of water and/or tailings
- Need to manage, store, and recycle process water
- Need to meet discharge water quality licence conditions
- Need to rehabilitate TSF in perpetuity

People, infrastructure and environment downstream are key risks

Conventional Tailings Disposal and Storage

Commonly held perception, supported by NPV approach, is that transporting tailings as a slurry to a dam is most economic

- Dewatering tailings to a paste or by filtration is perceived to be too expensive
- Reduced storage volume occupied by tailings paste or filter cake, and relative ease of capping are discounted, as is potential for a higher level future land use
- Cost of rehabilitating resulting soft and wet tailings is discounted and not considered to be significant
- Few TSFs have been rehabilitated, due to difficulty and expense of capping “slurry-like” tailings, particularly at a time when mine is no longer producing revenue

Key Causes of Tailings Dam Failures and Industry Threats

- Most tailings dams that fail have **marginal stability**
- Most tailings dam failures involve “**water**”, making drainage, clay cores and water management of key importance
- Many tailings are **potentially liquefiable**
- Another cause can be a **weak** (often unidentified, possibly moving from over- to normally-consolidated on progressive raising) **foundation layer**
- **Industry threats** are coming from:
 - Investors; e.g., Church of England
 - Insurers
 - Regulators; e.g., outlawing upstream construction, which happened in Chile following earthquake-induced failures in 1965, and will follow in Brazil (wet climate and failures)

Susceptibility of Tailings to Liquefaction

- Earthquake-induced liquefaction susceptibility:
 - Fine-grained sandy or silty sand tailings – ✓
 - Loose (brittle [collapse beyond peak], contractive [more so at higher stress]) state – ✓
 - Near-saturated – ✓
 - Earthquake magnitude > 5.5 and peak ground acceleration >0.13g – ?Static or flow liquefaction, triggered by:
 - Loss of containment due to dam/upstream raise instability
 - Overtopping and erosion of dam/upstream raise
 - Rise in phreatic surface due to heavy rainfall or fresh tailings
 - Pore water pressure increase due to dam raise, recharge or blocked drains – Undrained loading

} Undrained unloading

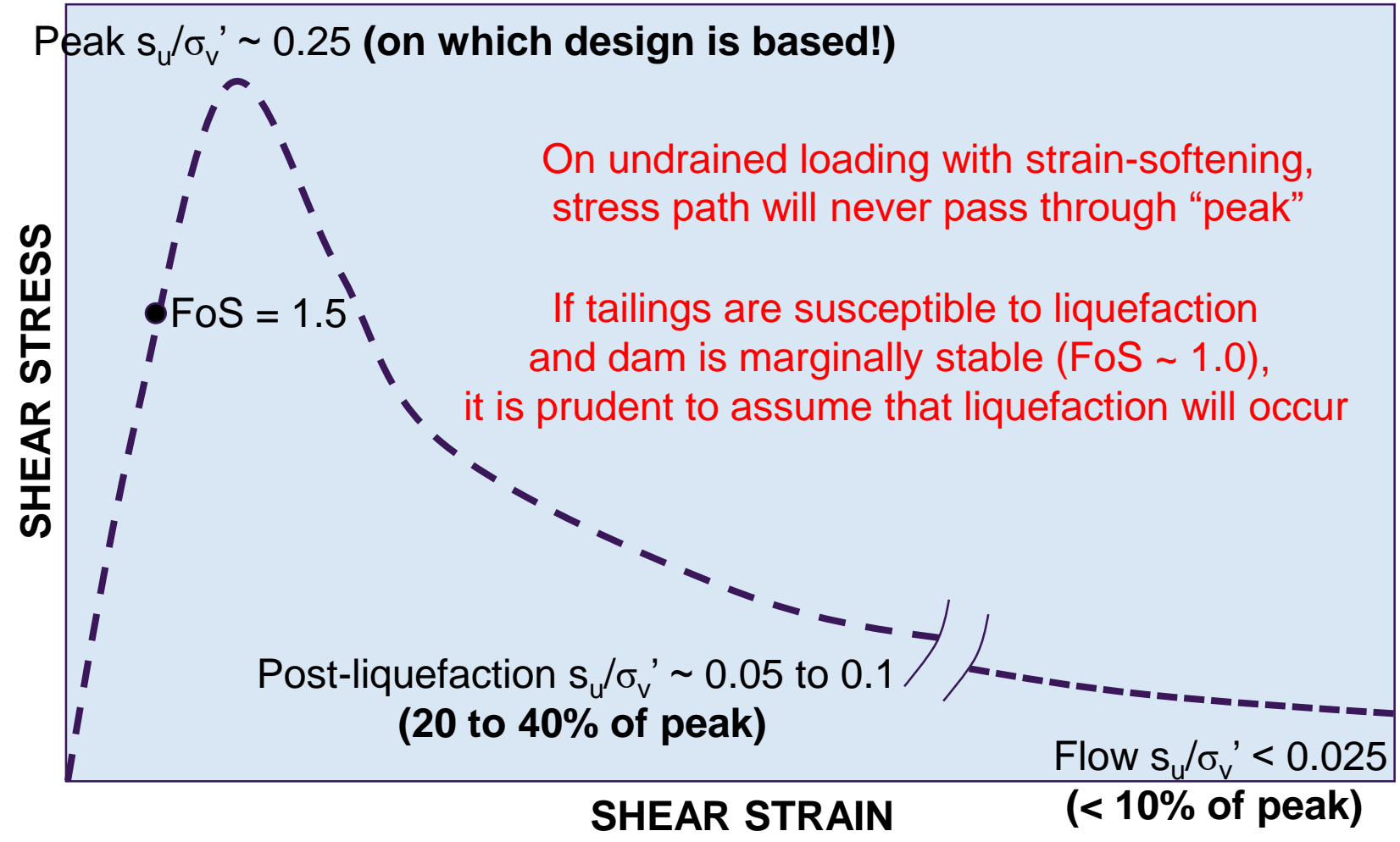
Susceptible tailings can behave in an undrained, contractive, strain-softening manner, and liquefy or flow

Susceptibility of Tailings to Liquefaction

- Moisture content \ll Liquid Limit suggest that tailings will not liquefy
- However, tailings state (loose or dense) *in situ* is difficult to determine, particularly if tailings are loose:
 - They can't be sampled at their *in situ* state, if at all (an indicator of liquefaction)
 - CPTu data plot in bottom left-hand corner of log-log SBT Chart, where few correlation data exist
 - Some use is made of SPT, with difficulty
 - Some use is made of “simple shear” testing, but samples may not be at *in situ* state

In absence of laboratory test data, liquefaction susceptibility and post-liquefaction shear strength may be estimated based on correlations between liquefaction case histories (notably mainly in natural, uncemented soils) and CPT cone resistance

Relative Peak, Post-Liquefaction and Flow Shear Strengths of Tailings

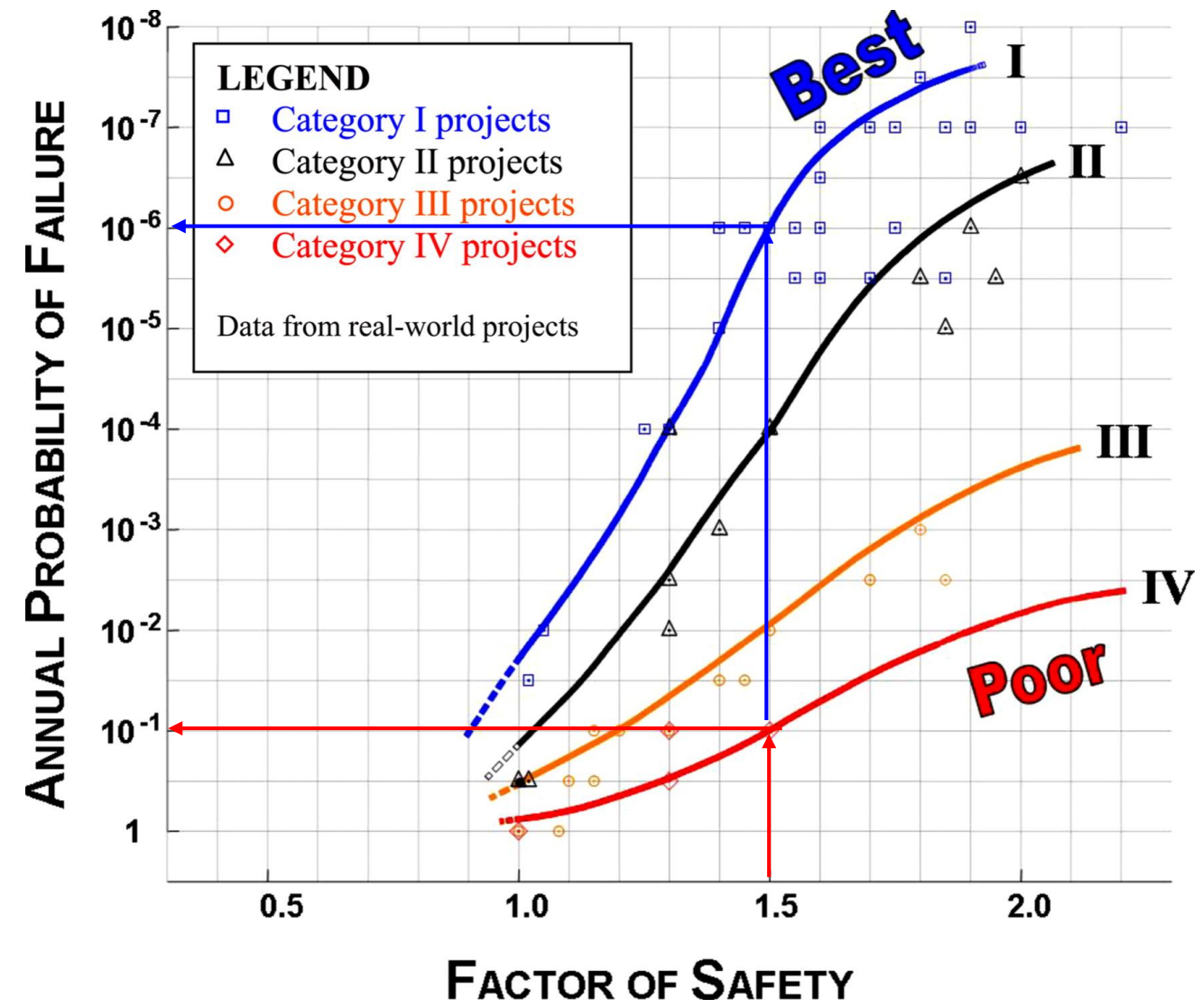


Commentary on Geotechnical Design Parameters

PARAMETER	TYPICAL RANGE
Angle of repose of waste rock	$37 \pm 3^\circ$ (depending on weathering and durability)
Annual Rainfall	$\pm 50\%$ of average annual rainfall
Design peak ground acceleration	Perhaps $\pm 20\%$ for operations to $\pm 50\%$ for closure?
Undrained cohesion	$\pm 50\%$
Undrained shear strength ratios: Peak Residual	0.25 0.05 to 0.10, or lower
Drained cohesion	Often assumed = 0, but suction on desiccation will induce some apparent cohesion
Drained friction angle	$\pm 3^\circ$ (~ 6° higher than angle of repose, implying a Factor of Safety = 1.24)
Unit weight	$\pm 10\%$ (typically has little impact)
Erosion loss	$\pm 100\%$ or greater?

What are error bars on calculated Factor of Safety? – Certainly >> 1 decimal place!

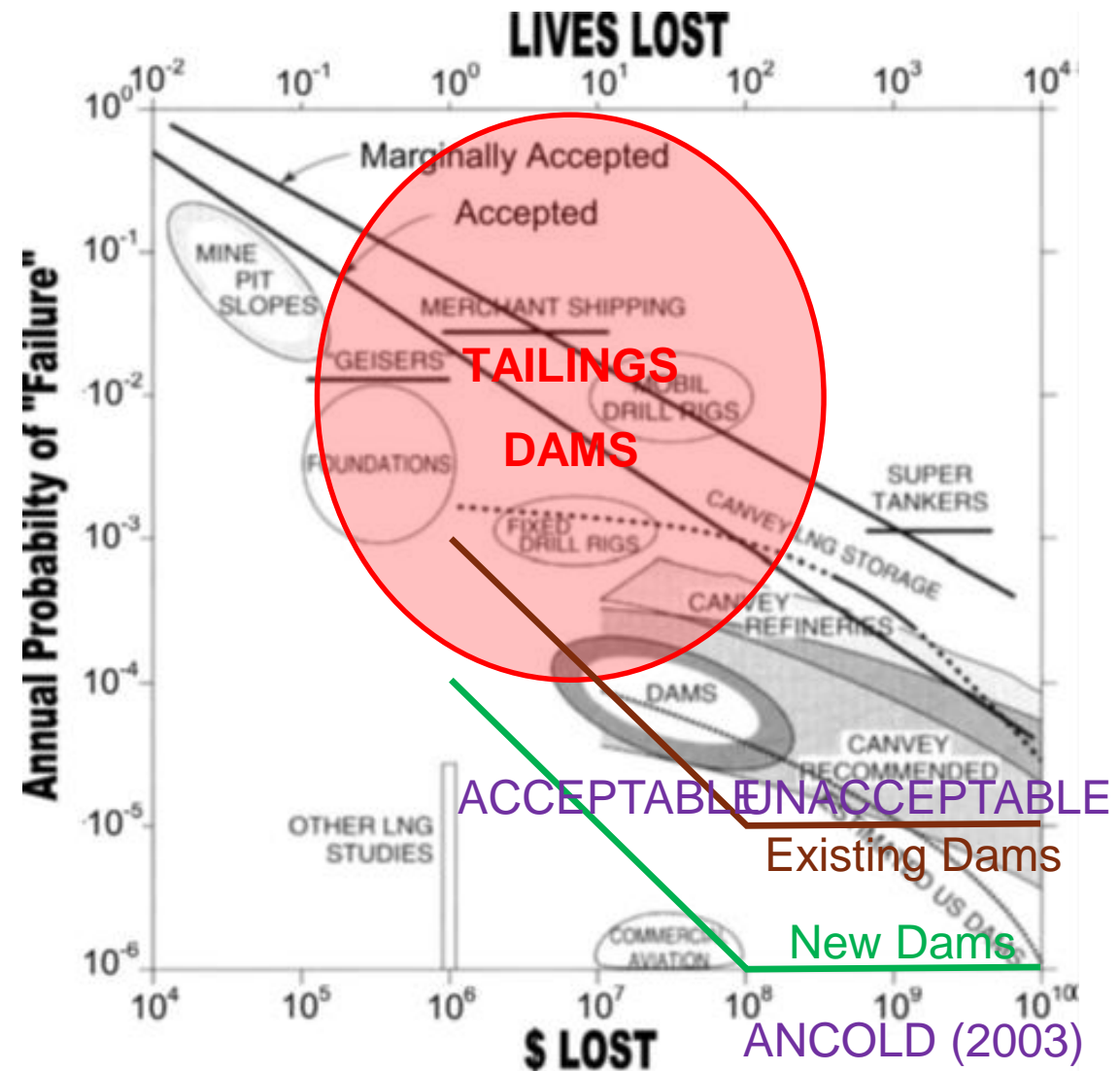
Annual P_f versus FoS (Silva, Lambe and Marr, 2008)



Way Forward

- Risk assessments of tailings dams are common-place
- Defining what is an “**acceptable**” risk level is difficult
- Ongoing rate of tailings dam failures is **unacceptable** (100 x that of water dams!)
- Approaches to tailings management need to improve
- Tailings minimisation and dewatering need to be pursued
- Design, construction, operation and closure of TSFs need > **reliability & resilience**
- **Monitoring and interpretation** of tailings dams needs to be more comprehensive, and in real-time, linked to **triggers**:
 - **Green**, for safe operation
 - **Amber**, requiring assessment by a Geotechnical Engineer
 - **Red**, initiating Emergency Response Plan

Acceptability of Tailings Dam Risk? (After Silva, Lambe and Marr, 2008)



Emerging Tailings Dam Surveillance Methods

METHOD

Drone surveillance

Nested VW Piezometers

V-notch weirs for monitoring flows

Settlement monuments and inclinometers

LiDAR (above water) and bathymetry (below water)

Slope Stability Radar

Satellite InSAR

Environmental monitoring

Future “Smart” geofabrics

COMMENTS

Aiding visual inspection and LiDAR

Recording and analysing phreatic surface and hydraulic gradient in real-time

Recording and analysing flows in real-time

For correlation with Radar and InSAR – Must be high resolution

For density estimation, and evaporation estimation

Potential “noise” issues due to short wavelength, rapid scanning, and lack of visible targets can reduce reliability

Fortnightly/weekly fly-over for line-of-sight deformations (potentially to ± 3 mm/year), wet spots and vegetation

For water quantity and quality

In tailings beach, or under upstream raises

Radar Frequencies and Wavelengths (Source: GroundProbe)

PARAMETER/BAND	L-BAND	S-BAND	C-BAND	X-BAND	Ku-BAND
FREQUENCY (GHz)	1-2	2-4	4-8	8-12	12-18
WAVELENGTH (cm)	30-15	15-7.5	7.5-3.75	3.75-2.5	2.5-1.67
COMMON USES	Radio/Mobile communications	Airport surveillance	Satellite communications	Wireless communications	Microwaves, affected by moisture
InSAR; e.g.	JERS ALOS PALSAR ALOS2		ESA Sentinel RADARSAT1/2	TerraSAR- XCOSMO- SkyMed	
	Not much used for InSAR		Long-term, large area deformation		
GROUND-BASED				GBRAR (Real)	GBSAR (Synthetic)
				Rapid, targeted scanning; e.g., of pits Application to tailings dams affected by moisture, vegetation and rapid scanning	

Monitoring to Provide Adequate Stability and Warning of Failure

Above all, design, operate and close a tailings dam ensuring an adequate FoS against geotechnical failure

- Avoid, where possible, people at risk downstream of a tailings dam
- Aim to provide at least 24 hours warning to evacuate any people downstream of a tailings dam
- Install redundant monitoring systems, recorded and analysed in real-time, and linked to triggers and responses, including:
 - Daily inspections for wet spots, cracking, deformation, etc., aided by drones
 - Phreatic surface and hydraulic gradient – Nested VW Piezometers confirmed by regular CPTu
 - Deformation – Combination of automated total station prisms, inclinometers, Radar (long wavelength), and InSAR
 - Flows and water quality monitoring – Automated V-notch weirs and sampling

Lessons Learned from Recent Tailings Dam Failures

- Common Threads:
 - Fatal tailings dam failures are more likely in developing countries
 - Most past and recent tailings dam failures involve dams constructed upstream
 - Split of failures is roughly equal between closed and operational dams
 - Split of liquefaction failures is roughly equal between seismic and static liquefaction
 - About half of recent tailings dam failures involve weak foundations
 - Cementation and suction provide apparent cohesion, which can be lost
 - A closed or inactive dam should not be considered safe unless monitoring indicates unsaturated conditions (i.e., drain down on cessation of tailings deposition, which would be expected in a dry climate, but may not occur in a wet climate)
 - Tailings dam failures occur very rapidly, with little or no warning
 - While mechanisms of failure can usually be identified retrospectively, triggers are more difficult to identify

Some Upcoming Meetings and Conferences

ANCOLD – Considering Liquefaction of Tailings Workshop – 12 November 2019

Perth – <https://www.ancold.org.au/?p=15322>

10th Australian Workshop on Acid and Metalliferous Drainage (AMD) 2020 – 23-

26 March 2020, Dubbo – <https://amdworkshop.com.au/home>

Mine Waste and Tailings (MWT) 2020 – 28-30 July 2020 in Brisbane –

<https://tailings.ausimm.com/> – Call for Abstracts open till end October 2019

International Conference on Acid Rock Drainage (ICARD) 2021 – 30 August to 3

September 2021, Brisbane – <https://www.inap.com.au/icard/>